



A model library for simulation and benchmarking of integrated urban wastewater systems



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ABSTRACT

This paper presents a freely distributed, open-source toolbox to predict the behaviour of urban wastewater systems (UWS). The proposed library is used to develop a system-wide Benchmark Simulation Model (BSM-UWS) for evaluating (local/global) control strategies in urban wastewater systems (UWS). The set of models describe the dynamics of flow rates and major pollutants (COD, TSS, N and P) within the catchment (CT), sewer network (SN), wastewater treatment plant (WWTP) and river water system (RW) for a hypothetical, though realistic, UWS. Evaluation criteria are developed to allow for direct assessment of the river water quality instead of the traditional emission based metrics (for sewer overflows and WWTP discharge). Three case studies are included to illustrate the applicability of the proposed toolbox and also demonstrate the potential benefits of implementing integrated control in the BSM-UWS platform. Simulation results show that the integrated control strategy developed to maximize the utilization of the WWTP's capacity represents a balanced choice in comparison to other options. It also improves the river water quality criteria for unionized ammonia and dissolved oxygen by 62% and 6%, respectively.

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Software availability

Name of the software:

BSM-UWS.

Developers:

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Programming language:

Matlab 13.0.

Software availability: The source code for the system-wide BSM can be obtained for free. Contact Dr Ulf Jeppsson, division of Industrial Electrical Engineering and Automation (IEA), Lund University, Box 118, SE-221 00 Lund, Sweden (ulf.jeppsson@iea.lth.se). The software is documented and interested readers will be able to reproduce the results summarized in this article, and then modify the software for their own purposes as well.

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1. Introduction

The main objective of integrated modelling is to link various sections of the urban wastewater system (UWS) (catchment (CT), sewer network (SN), wastewater treatment plant (WWTP) and receiving water system (RW)) together to provide a unified platform for design and analysis of wastewater infrastructures in urban areas (Benedetti et al., 2013). Such a tool enables direct evaluation of UWS dynamic performance (or of individual sections) based on river water quality instead of relying on traditional emission based evaluation. Significant progress has been made in the field of integrated modelling ever since it was first proposed by Beck (1976) (e.g. Fronteau et al., 1997; Rauch et al., 2002; Muschalla et al., 2009; Benedetti et al., 2013; Bach et al., 2014). It is now well established that optimization of sub-system performance (SN or WWTP) does not necessarily lead to improvements in river quality (Rauch and Harremoës, 1999) and a more holistic approach is required (Lijklema et al., 1993). Although research has highlighted the need of integrated modelling for a long time, a strong incentive for receiving water quality based evaluation of UWS performance has been provided by the EU Water Framework Directive, which calls

Nomenclature

$(P)_{\text{eff,WWTP}}$	Pollutant (P) total load at WWTP effluent (kg) ($P = \text{BOD}_5, \text{COD}, \text{TSS}, \text{TKN}, P_{\text{org}}$ and P_{inorg})	PC	Primary clarifier
$(P)_{\text{EMC}}$	Pollutant (P) EMC (g/m^3)	P_{inorg}	Inorganic phosphorus
$(P)_{\text{in,WWTP}}$	Pollutant (P) total load at WWTP inlet (kg)	PO_4	Phosphate
$(P)_{\text{ovf}}$	Pollutant (P) total load in overflow (kg)	P_{org}	Organic phosphorus
AER1	Aerobic reactor 1	Q_{intr}	Internal recirculation rate (m^3/d)
AER2	Aerobic reactor 2	$Q_{\text{in,WWTP}}$	Inflow to WWTP (m^3/d)
AER3	Aerobic reactor 3	$Q_{\text{max,BP1}}$	Maximum flow at bypass 1 (m^3/d)
ANAER1	Anaerobic reactor 1	$Q_{\text{max,BP2}}$	Maximum flow at bypass 2 (m^3/d)
ANAER2	Anaerobic reactor 2	$Q_{\text{max,ST2}}$	Maximum throttle flow for ST_2 (m^3/d)
ANOX1	Anoxic reactor 1	$Q_{\text{max,ST5}}$	Maximum throttle flow for ST_5 (m^3/d)
ANOX2	Anoxic reactor 2	$Q_{\text{max,ST6}}$	Maximum throttle flow for ST_6 (m^3/d)
BOD_5	5-day biological oxygen demand	$Q_{\text{pump,ST1}}$	Pumping rate at ST_1 (m^3/d)
BP_1	Bypass 1 (before primary clarifier)	$Q_{\text{pump,ST4}}$	Pumping rate at ST_4 (m^3/d)
BP_2	Bypass 2 (after primary clarifier)	Q_r	Sludge recycle rate (m^3/d)
$C_{\text{max,NH}_3}$	Hourly maximum concentration for unionized ammonia ($\text{g N}/\text{m}^3$)	$Q_{\text{throttle,ST4}}$	Maximum throttle flow for off-line tank ST_4 (m^3/d)
$C_{\text{min,DO}}$	Hourly minimum concentration for dissolved oxygen (g/m^3)	Q_w	Sludge wastage rate (m^3/d)
COD	Chemical oxygen demand	RST	Rainwater storage tank
COD_{part}	Particulate COD	$\text{RW}_{(i)}$	River water stretch i
COD_{sol}	Soluble COD	$\text{SC}_{(i)}$	Sub-catchment i
DO	Dissolved oxygen	Sec.C	Secondary clarifier
EMC	Event mean concentration (g/m^3)	$S_{\text{NH}_4,\text{RW16}}$	Sensor measurement for ammonia (NH_4^+) at river stretch 16 ($\text{g N}/\text{m}^3$)
EQI	Effluent quality index (kg pollution units/d)	$S_{\text{O}_2,\text{AER2}}$	Sensor measurement for oxygen concentration at aerobic reactor 2 (AER2) (g/m^3)
h_{ST_i}	Height of storage tank i	$\text{ST}_{(i)}$	Storage tank i
IQI	Influent quality index (kg pollution units/d)	$S_{\text{TSS,eff}}$	Sensor measurement for total suspended solids (TSS) at WWTP effluent ($\text{g N}/\text{m}^3$)
$K_{L\alpha\text{AER}(i)}$	Oxygen transfer coefficient for aerobic reactor i (d^{-1})	$T_{\text{exc,DO}}$	Yearly exceedance duration for dissolved oxygen in river (h)
MLSS	Mixed liquor suspended solids	$T_{\text{exc,NH}_3}$	Yearly exceedance duration for unionized ammonia in river (h)
NH_3	Unionized ammonia	TKN	Total Kjeldahl nitrogen
NH_4^+	Ammonia	T_{ovf}	Yearly overflow duration (days/year)
NO_3^-	Nitrate	TSS	Total suspended solids
N_{ovf}	Yearly overflow frequency (events/year)	V_{ovf}	Yearly overflow volume (m^3)
$\text{OVF}_{(i)}$	Overflow at location no. i	WWTP_{eff}	Wastewater treatment plant effluent discharge into the river system
QQI	Overflow quality index (kg pollution units/d)		

for achieving “good ecological and chemical status in all rivers” (EU, 2000). Today, tools for integrating sub-system models running on different platforms exist (Gregersen et al., 2007) and case studies illustrating their usage are available in the literature (Reußner et al., 2008; Van Assel et al., 2010). Commonly used commercial simulation software packages (e.g. SIMBA (ifak, Germany), WEST (DHI, Denmark)) provide libraries that allow users to develop system-wide models on a single platform. Additionally, several modelling libraries are developed by various researchers (e.g. Schütze, 1998; Achleitner et al., 2007; Mannina, 2005; Freni et al., 2010b; Willems and Berlamont, 2002).

Design and evaluation of (local/global) control strategies are two of the major areas where integrated models showed their full potential (e.g. Schütze et al., 2002; Meirlaen et al., 2002; Langeveld et al., 2013; Seggelke et al., 2005). Some of the studies were extremely successful, provided a lot of scientific inspiration for further control development and clearly demonstrated the benefits of using integrated approaches. Nevertheless, the evaluation/comparison of these control strategies, either real or model-based, is difficult. This is due to a number of reasons, including: i) variation in the characteristics of the UWS (catchment layout, sewer and WWTP design, river water quality etc.); ii) differences in the underlying models for describing the hydraulic, biological and

physico-chemical processes in the UWS; and iii) the lack of a common evaluation method to compare the results. Hence, the objective comparison of the reported strategies has been a challenge.

A similar problem in the WWTP modelling community is addressed by using Benchmark Simulations Models (BSMs). Several researchers working under the umbrella of the International Water Association (IWA) benchmarking task group developed different benchmarks (BSM1, BSM1_LT, BSM2) to facilitate an unbiased comparison of control strategies in WWTPs (Copp, 2002; Rosen et al., 2004; Nopens et al., 2010). These BSMs consist of pre-defined layouts, process models, sensor/actuator models, influent characteristics and evaluation criteria (Gernaey et al., 2014). They have seen huge success (500 + publications) and are widely accepted in the research/practice community (Jeppsson et al., 2013). Similar efforts in the urban drainage community have been made where pre-defined sewer system layouts are used for comparing various real time control strategies and static design options (Borsányi et al., 2008; Schütze et al., 2015). Besides the original objective of comparing control strategies (Stare et al., 2007; Flores-Alsina et al., 2008; Sweetapple et al., 2014), the different tools developed by the BSM group are also used to develop better solvers (Rosen et al., 2008; Flores-Alsina et al., 2015), model

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